

Effect of different climatic conditions on the degradation of historical masonry in the Czech Republic

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Abstract

The effect of different climatic conditions on the degradation of historical masonry in the Czech Republic is studied. Several different sandstone and several different locations in the Czech Republic are selected for this analysis. The effect of environmental conditions is evaluated using various damage functions. Based on the damage function values, the most severe and most favorable locations are identified, together with the most resistant sandstone in terms of hygrothermal loads.

Keywords: sandstone, historical masonry, damage function, climatic conditions.

1 Introduction

Historical buildings are an integral part of contemporary cityscapes. The age of these buildings ranges from hundreds to thousands of years. With such a high age, it is inevitable to renovate these buildings in certain periods. The renovation processes are mostly controlled by monument care authorities, which insist on preservation of original treatments, technologies and materials from the period of the building's rise. Naturally, there is a significant effort of the designers and renovators to maximize the renovation periods. In order to ensure the maximum service life of the renewed construction, it is necessary to analyze the environmental conditions the constructions are exposed to. Such an analysis must be a part of the process of renovation design. The choice of proper material must be done with respect to the nature of the building; last but not least, the environmental conditions must be taken into consideration as well. Improper



choice of the material or material composition regarding to the environment of given location may lead to a significant shortening of the service life period [1–4].

The assessment of service life of historical buildings is not an easy task as many aspects and various damage effects are to be taken into account. Such assessment cannot be done on the basis of empirical knowledge. In order to obtain serious results, it is necessary to utilize some advanced techniques, for example the computational modelling [5, 6].

Computational modelling became very strong and powerful tool in many fields of engineering. In civil engineering, computational modelling is used within the frame of design and development of new materials, as well as for simulation and analysis of existing constructions. The biggest advantage of this kind of modelling is the fact that the studied building or its part does not have to physically exist and still, the engineers are able to study its performance from different points of view. In this way, the potential weak points of the construction may be revealed before the construction is physically built up and thus, the economic risk may be controlled more effectively, which is of double importance in case of renovation works.

Computational simulations are able to provide the hygrothermal performance of historical buildings, i.e., moisture and temperature fields within the construction, as a function of time [2]. On the basis of such performance, it is possible to seriously estimate the service life of the construction or to maximize the effect of renovation measures. The computational modelling allows effective scheduling of the renovation periods as well.

In this paper, the computational simulation is used in order to calculate the possible degradation of the historical masonry made of several different stones and located under different climatic loads typical for the Czech Republic.

2 Computational analysis

2.1 Mathematical model

For the investigation of hygrothermal performance of studied constructions a modified Künzel's mathematical model was used [7]. The model is able to distinguish more precisely between the liquid and gaseous phase of moisture transport and thus brings higher accuracy to the calculations [8]. Balance equations are expressed as:

$$\frac{d\rho_v}{d\varphi} \frac{\partial\varphi}{\partial t} = \operatorname{div} \left[\left(B(D_w \rho_w \frac{dw}{d\varphi}) + A(\delta_p p_s) \right) \operatorname{grad}\varphi + \left(\delta_p \varphi \frac{dp_s}{dT} \right) \operatorname{grad}T \right] \quad (1)$$

$$\frac{dH}{dT} \frac{\partial T}{\partial t} = \operatorname{div}(\lambda \operatorname{grad}T) + L_v \operatorname{div}[\delta_p \operatorname{grad}(\varphi p_s)], \quad (2)$$

where ρ_v is the partial density of moisture, φ is the relative humidity, δ_p is the permeability of water vapor, p_s is the partial pressure of saturated water vapor, H is the enthalpy density, L_v is the latent heat of evaporation, λ is the thermal

conductivity, T is the temperature, D_w is the total capillary transport coefficient, ρ_w is the density of water and w is the moisture content. The transition function assigns the weight to parameters A and B (being within the range of 0–1) with the balance $A + B = 1$ valid for every point of the curve.

The satisfactory combination of power functions reads [8]:

$$B = 32 \left[\left(\frac{1}{\varphi_2 - \varphi_1} \right) (\varphi - \varphi_1) \right]^6, \text{ for } \varphi = \langle 90; 93.8 \rangle, \quad (3)$$

$$B = 1 - 32 \left[\left(\frac{1}{\varphi_2 - \varphi_1} \right) (\varphi_2 - \varphi) \right]^6, \text{ for } \varphi = \langle 93.8; 97.6 \rangle, \quad (4)$$

where $B = 0$ for $\varphi < 90\%$, $B = 1$ for $\varphi > 97.6\%$, $B \in \langle 0; 1 \rangle$ according to (3) and (4) and $A = 1 - B$.

The solution of the transport problem leads to necessity of solving partial differential equations. These equations were solved by finite element approach using computer code SIFEL [9].

2.2 Material parameters

In order to demonstrate the effect of different climatic conditions on the degradation of historical masonry in the Czech Republic, the hygrothermal performance of five different sandstone masonries typical for historical buildings were investigated. The thickness of the studied load-bearing construction was 600 mm. The studied sandstones originate from the quarries Božanov (SB), Hořice (SH), Kocbeře (SK), Libnava (SL) and Záměl (SZ). The mineralogical compositions of the sandstones are shown in Table 1. The basic physical, heat and moisture transport and storage properties are given in Table 2, where ρ is the bulk density, ψ the porosity, c the specific heat capacity, $\mu_{dry\ cup}$ the water

Table 1: Mineralogical composition of studied sandstone.

	SH	SB	SK	SL	SZ
Quartz	73%	78%	83%	71%	79%
Feldspar	9%	–	–	–	4%
K-feldspar	–	13%	2%	8%	–
Glauconite	–	–	–	3%	9%
Muscovite	1%	1%	1%	–	1%
Rutile	–	–	1%	–	–
Tourmaline	1%	1%	1%	–	–
Plagioclase	–	13%	–	–	–
Titanite	–	–	–	1%	–
Zircon	1%	1%	–	1%	–
Biotit	–	1%	–	–	–

vapor diffusion resistance factor in dry conditions, $\mu_{wet\ cup}$ the water vapor diffusion resistance factor in wet air, λ_{dry} the thermal conductivity in dry conditions, λ_{sat} the thermal conductivity in water saturated conditions, κ_{app} is the apparent moisture diffusivity. All the parameters were measured in the laboratories of the Department of Materials Engineering and Chemistry, Faculty of Civil Engineering, Czech Technical University in Prague and stored in the material database.

Moisture diffusivity, specific heat capacity and thermal conductivity as a function of moisture content together with sorption isotherms of studied materials were presented in [10, 11].

Table 2: Material parameters of studies sandstones [10, 11].

	SH	SB	SK	SL	SZ
ρ (kg/m ³)	2004	2154	2228	2191	2076
ψ (%)	21.6	16.1	16.1	17.9	22.8
c (J/kgK)	712	672	760	628	694
$\mu_{dry\ cup}$ (-)	11.59	13.38	12.7	11.6	11.8
$\mu_{wet\ cup}$ (-)	5.77	7.18	7.4	7.4	6.9
λ_{dry} (W/mK)	2.58	2.35	3.53	2.71	2.10
λ_{sat} (W/mK)	3.76	4.54	5.21	4.64	3.88
κ_{app} (m ² /s)	4.14e-8	2.50e-7	7.17e-7	5.76e-8	7.76e-7

2.3 Boundary conditions

Boundary conditions in the interior were set constant as 21°C and 55% relative humidity. On the exterior side, the climatic load in a form of Test Reference Year (TRY) of five different locations was applied, namely Prague-Karlov, České Budějovice, Tušimice, Košetice, Pec pod Sněžkou and Brod nad Dyjí (see Table 3). The TRYs were obtained from the Czech Hydrometeorological Institute (CHMI) which is an official authority for climatology, hydrology and atmosphere quality in the Czech Republic. The weather data obtained from CHMI as well as other sources are stored in weather database [12]. The map of

Table 3: Studied localities.

	Locality	Altitude
1	České Budějovice	385 m n.m.
2	Košetice	534 m n.m.
3	Pec p. Sněžkou	824 m n.m.
4	Praha – Karlov	232 m n.m.
5	Tušimice	322 m n.m.
6	Brod nad Dyjí	177 m n.m.

selected locations is shown in Fig. 1. The monthly values of relative humidity, temperature and yearly amount of precipitation typical for studied location are shown in Figs 2–4.

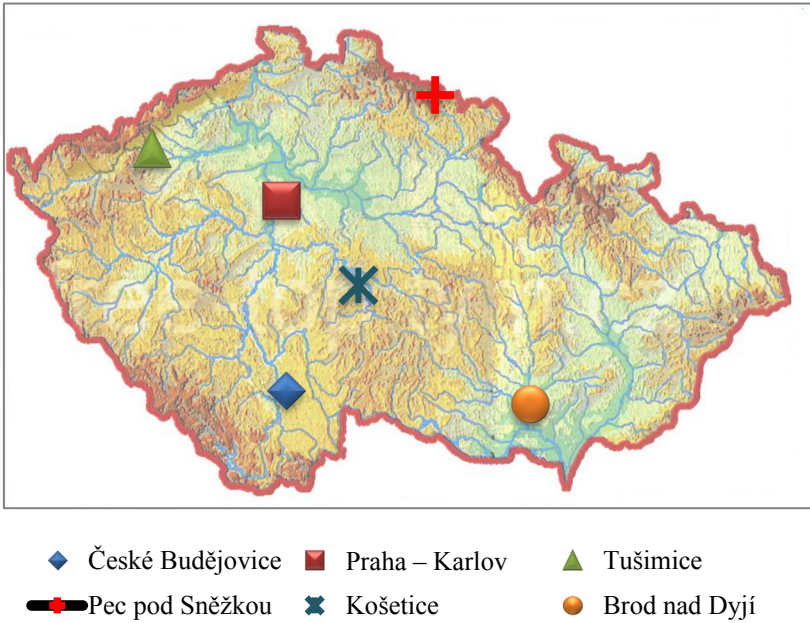


Figure 1: The map of the Czech Republic with selected localities.

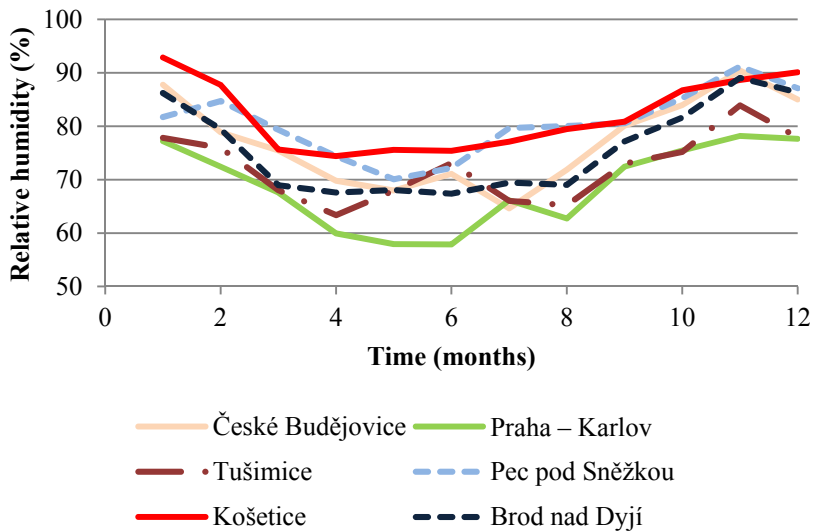


Figure 2: Average monthly values of relative humidity.

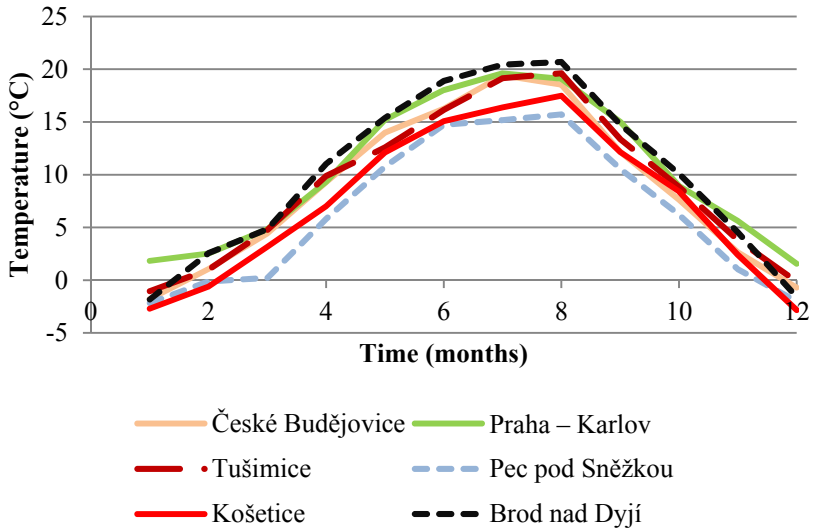


Figure 3: Average monthly values of temperature.

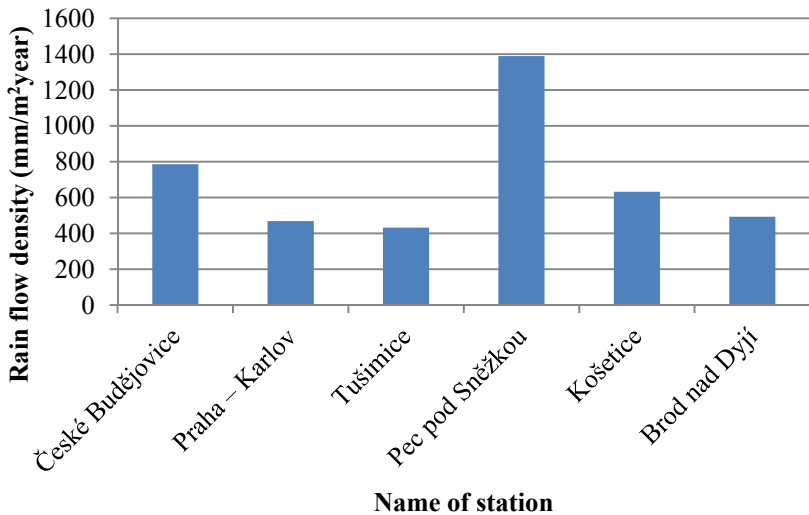


Figure 4: Yearly amount of precipitation.

3 Results and discussion

The results of computational simulations performed by SIFEL code are presented below. All the computer simulations were accomplished for 10 years in order to ensure steady conditions in the studied walls.

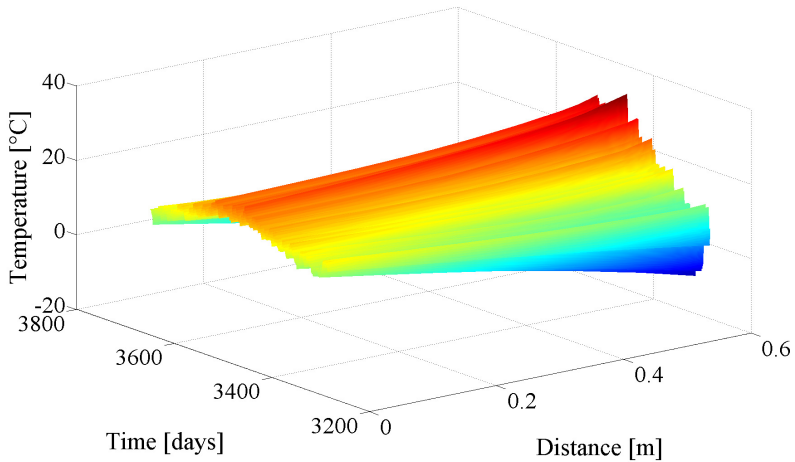


Figure 5: Temperature distribution in SL sandstone as a function of time and space (location Pec p. Sněžkou).

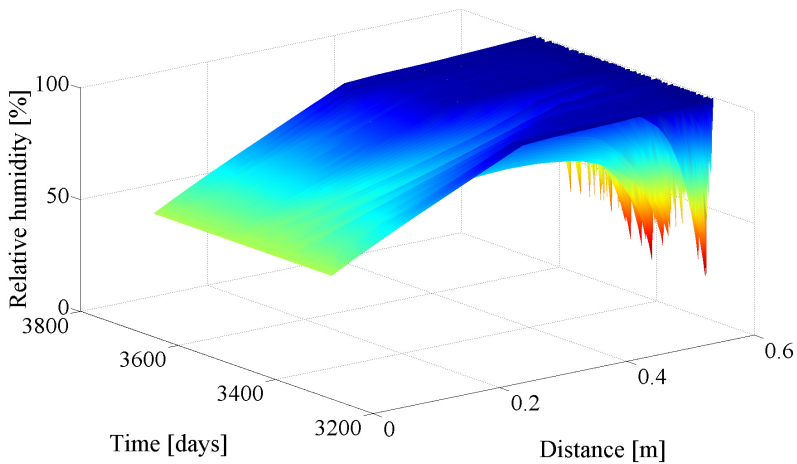


Figure 6: Relative humidity distribution in SL sandstone as a function of time and space (location Pec p. Sněžkou).

Just for illustration, temperature and relative humidity as a function of time and space during the last, 10th year of the simulation are shown in Figs 5 and 6. The hygrothermal response of each studied construction was analyzed in order to determine the effect of the studied environment on the degradation of historical masonry. During the service life, the constructions are exposed to a range of physical, chemical and mechanical stresses. The amount of moisture that is retained in the construction significantly affects the durability of the construction. However, the amount of moisture in the construction does not result from environmental conditions only, the material composition plays significant role as well [6, 13]. The ability of construction materials to transfer loads depends on the magnitude of applied stresses, moisture contents and temperatures. In order to quantify the damage that is caused to the construction, it is necessary to employ the damage function. The choice of particular damage function should be done with respect to the investigated material, its characteristic type of damage and the ambient environment in order to obtain credible results. There are described several damage functions in the literature, for example, Time-of-Wetness (TOW) [14], RHT-Index [15], Mold-Growth-Index [16, 17], Winter index [18] etc. In this paper, RHT-Index, TOW and Winter index was used to quantify the damage caused to the investigated constructions. The brief description of selected damage functions is given below.

TOW is one of the standard damage functions. TOW was used in various engineering tasks in various ways. In this paper, the TOW is calculated as a time in hours during a year where both the temperature and relative humidity were above the prescribed levels. The value of TOW ranges between 0 and 8760. The prescribed levels in the research were set to 0°C and 95% of relative humidity.

The RHT-Index is similar to TOW, but instead of calculating only time when the prescribed conditions are met, RHT-Index places emphasis on the rate of exceeding the prescribed values of temperature and relative humidity. RHT-Index is calculated only if $T > T_0$ and $RH > RH_0$ according to

$$RHT = \sum (T - T_0) \cdot (RH - RH_0), \quad (5)$$

where $T_0 = 0^\circ\text{C}$ and $RH_0 = 95\%$ are the prescribed values.

Winter index (WI) damage function was designed for such locations where the typical damage is caused by alternation of freezing and thawing periods:

$$WI = \sum (T - T_0) \cdot (RH - RH_0). \quad (6)$$

In principle, the method is based on RHT-Index calculations, but the definition of prescribed values is different. As the WI should describe the induced frost in the building envelopes, the T_0 is equal to an equilibrium temperature of water-ice phase change and RH_0 corresponds to the maximum hygroscopic moisture content of porous building materials. Due to safety reasons, the values of T_0 and RH_0 were set to 0°C and 95% respectively, despite the fact that freezing point of water in porous building materials is lower than 0°C and the condensation of water vapor occurs when the relative humidity is above 97%. The WI is calculated only if $T < T_0$ and $RH > RH_0$. The value of WI

is always negative, the lower value WI returns, the more severe damage conditions were in the studied material.

All of the studied sandstone masonries were analyzed using above mentioned damage functions. The point of investigation was placed 2 mm under the exterior surface of the masonry. The analysis was done for the last year of the simulation. Moreover, for each studied wall the maximum freezing depth (from exterior surface) where the possible freezing of water may occur was identified (MFD).

The analysis presented in Table 4 shows that the most severe environmental conditions were typical for location of Pec p. Sněžkou. The values of WI for that

Table 4: Analysis of hygrothermal response of studied stone masonries.

		Materials				
Location	Damage	SH	SB	SK	SL	SZ
České Budějovice	WI	-390	-338	-390	-456	-109
	TOW	1192	1159	1149	1222	977
	RHT	10328	10231	11748	11515	5704
	MFD	150 mm	200 mm	100 mm	150 mm	200 mm
Košetice	WI	-78	-57	-49	-86	-9
	TOW	1331	1290	1308	1380	964
	RHT	13784	13559	15940	15767	4720
	MFD	200 mm	75 mm	150 mm	200 mm	40 mm
Pec p. Sněžkou	WI	-903	-790	-776	-1045	-210
	TOW	2561	2407	2612	2770	2362
	RHT	60112	61131	75832	73331	25561
	MFD	200 mm	125 mm	200 mm	250 mm	100 mm
Praha - Karlov	WI	-130	-116	-142	-171	-45
	TOW	110	106	102	117	63
	RHT	724	712	817	880	299
	MFD	20 mm	50 mm	15 mm	20 mm	20 mm
Tušimice	WI	0	0	0	0	0
	TOW	250	243	238	263	189
	RHT	2630	2613	3043	3005	1569
	MFD	0 mm	0 mm	0 mm	0 mm	0 mm
Brod n. Dyjí	WI	-177	-136	-123	-195	-43
	TOW	958	931	952	993	780
	RHT	11746	11574	13533	13372	6092
	MFD	150 mm	100 mm	50 mm	200 mm	100 mm

location ranged from -210 to -1045, which was significantly higher than for other localities. The same rule applied for other studied damage parameters in this location. Contrary to Pec p. Sněžkou, the location of Tušimice proved the lowest damage conditions for historical masonry. The water-ice phase change in the material was practically disproved, which was affirmed by zero values of WI and MFD. On the other hand, the values of RHT were above the average which proved propitious conditions in terms of frost resistance during the winter period.

By comparing the results obtained for the studied sandstones it can be concluded that the best hygrothermal performance and durability was achieved with the sandstone from Záměl quarry (SZ). The worst performance could be registered in the case of sandstone from Libnava quarry (SL).

4 Conclusions

In this paper, the hygrothermal analysis of five selected sandstones under five different environments was carried out. Such analysis allows studying the effect of different climatic conditions on the degradation of historical masonry in the Czech Republic. For the analysis of hygrothermal response of studied constructions several damage functions were used. The result of that analysis proved significant impact of the environment on degradation of historical masonry. More to that, the analysis can be used as a comparative study of different kinds of sandstones.

In order to make the analytical results more valuable, it would be preferable to implement more damage aspects or to involve coupled hygro-thermo-mechanical model. Such a model allows investigation of crack formations which have significant impact on further hygrothermal performance. Having hygro-thermo-mechanical model, the estimation of service life may be done more precisely, which can be subsequently included as one of the evaluation method for investigation of the effect of environmental conditions on the degradation of historical masonry.

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